



Infrastruct CS Ltd

Consulting Civil Engineers

Infrastruct CS Ltd
The Stables,
High Cogges Farm,
High Cogges,
Witney,
Oxon,
OX29 6UN
Tel: 01993 709709
Email: info@infrastructcs.co.uk
Web: www.infrastructcs.co.uk

27th July 2026

REF: 6314-MOLY-ICS-XX-RP-C-07.002

NEW TRANSFORMER – MOLLY MILLARS LANE

Surface Water Drainage Strategy & Compliance Statement

Site Location: Intersurgical Ltd, Molly Millars Ln, Wokingham RG41 2RZ

Planning Reference: 251907

Condition: Pre-Commencement Drainage Strategy (Surface Water Run-off) Condition 4

1. Executive Summary & Core Approach

This strategy outlines the surface water management for the on-site reordering of infrastructure. The works comprise the removal of a 10.5 m² smoking shelter and the installation of a new 10.0 m² transformer platform, along with associated paving of the area previously occupied by the shelter, and creating of soft-landscaping.

To ensure the development complies with Local Plan policies CC09 and CC10, the strategy achieves a net reduction in impermeable footprint, introducing localised soft landscaping to act as level for level floodplain compensation.

2. Impermeable Area Balance & Peak Run-Off Rates

The development results in a minor, positive net reduction in impermeable surfaces. Because the post-development impermeable footprint is smaller than the brownfield state, peak surface water run-off rates will naturally sit below or equal to existing brownfield condition without requiring artificial flow-restriction attenuation.

3. Specific Infrastructure & Run-Off Controls

3.1. Raised Transformer Platform & Additional Paving

Run-off from the new 10m² transformer platform and its operational paving will be managed closely at the source:

- **Interception:** A new surface water gully will be installed immediately adjacent to the new infrastructure to capture localised sheet-flow runoff from the platform and additional paving.
- **Continuity of Outfall:** To maintain historical drainage, this new gully will be routed directly into the lateral serving the site gully that originally served this specific area. This ensures absolute continuity of flows and prevents any new downstream connection points or hydraulic surcharging.

3.2. Regraded Soft Landscaping (Flood Compensation)

A dedicated 26m² area will be converted to soft landscaping to serve as localised flood compensation:

- **Self-Draining Design:** This regraded area will be installed as soft landscaping to maximise its ability to self-drain, utilising topsoil depths and planting that maximise natural infiltration and transpiration.
- Based on the reduction of drained area, the flows from the 10.0m² transformer, 15m² replacement paving and 26m² soft landscape can be expected to be 2.4 l/s for a 1 in 100 year event. The previously 100% impermeable area of 51m² could be expected to discharge at 3.9 l/s, a reduction in flows of 38%.

Directors:

R.J. White B.Sc (Hons) C.Env C.WEM I.Eng MCIWEM, A. J. Griffiths BEng (Hons) MCIHT
N. Jones, M. Blanco M.Eng CEng MICE Registered Office: Jamesons House, Compton Way, Witney,
OX28 3AB - Registered in England and Wales 07521130, VAT No: 111335958





Existing Drained Area = 51 m ²	Proposed Drained Area = 25m ²
1 in 1 year flows = 0.9 l/s	1 in 1 year flows = 0.5 l/s
1 in 2 year flows = 1.2 l/s	1 in 2 year flows = 0.7 l/s
1 in 30 year flows = 2.2 l/s	1 in 30 year flows = 1.3 l/s
1 in 100 year flows = 2.8 l/s	1 in 100 year flows = 1.7 l/s
1 in 100 +40%CC = 3.9 l/s	1 in 100 +40%CC = 2.4 l/s

4. Proposed Method of Surface Water Disposal

In line with the statutory SuDS hierarchy, the disposal method maintains the established site framework:

1. Infiltration will be maximised through the 26m² self-draining soft landscape, which filters and infiltrates water directly into the ground.
2. Infiltration cannot be the only method of drainage due to the local clay soils. For this reason, surface water from the hardstanding/transformer area is intercepted by the new gully and directed via the original outfall network, reducing the brownfield flow profile without increasing discharge volume. Waters from this connection point will be routed to the adjacent ditch, and from there to the Emm Brook

5. Maintenance Program

To satisfy the lifetime maintenance requirement of the planning condition, the following targeted maintenance regime is established:

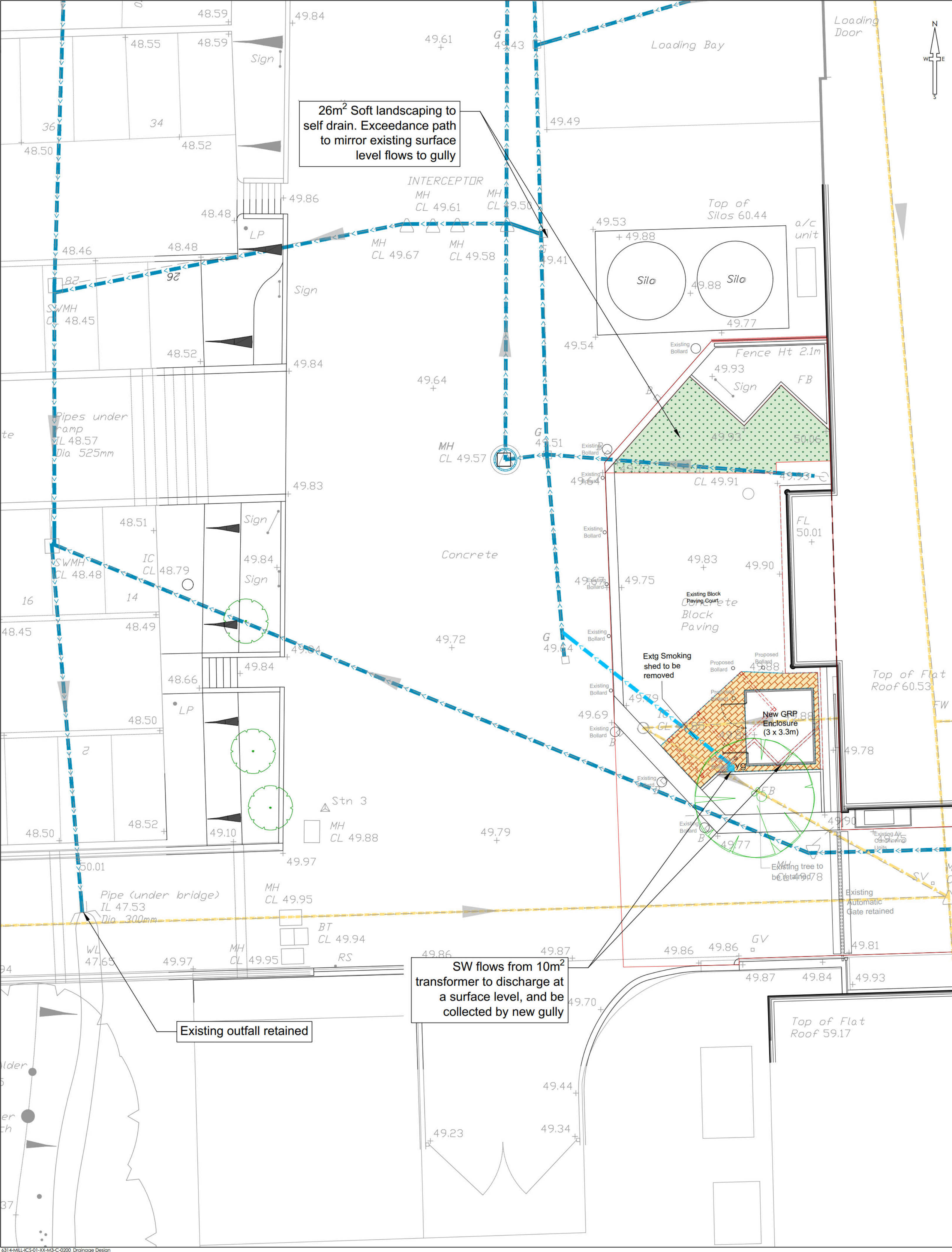
1. Gully and Catchpit Inspection: Quarterly & After Major Storms.
Inspect both the newly installed gully and the original receiving gully. Clear any silt, leaves, or debris to maintain full hydraulic capacity and continuity of flow.
2. Landscape Area Management: Bi-Annually (Spring / Autumn) & After Major Storms.
Inspect the 26m² self-draining soft landscape area. Clear dead vegetation, de-compact soil if signs of surface pooling occur, and ensure the area remains free of invasive weeds to protect its natural infiltration rates.

R. J. WHITE
BSc (Hons) CEnv CWEM IEng MCIWEM
DIRECTOR

A. J. GRIFFITHS
BEng (Hons) MCIHT
DIRECTOR



Appendix A – Drainage Layout and Catchment Areas



DRAINAGE NOTES

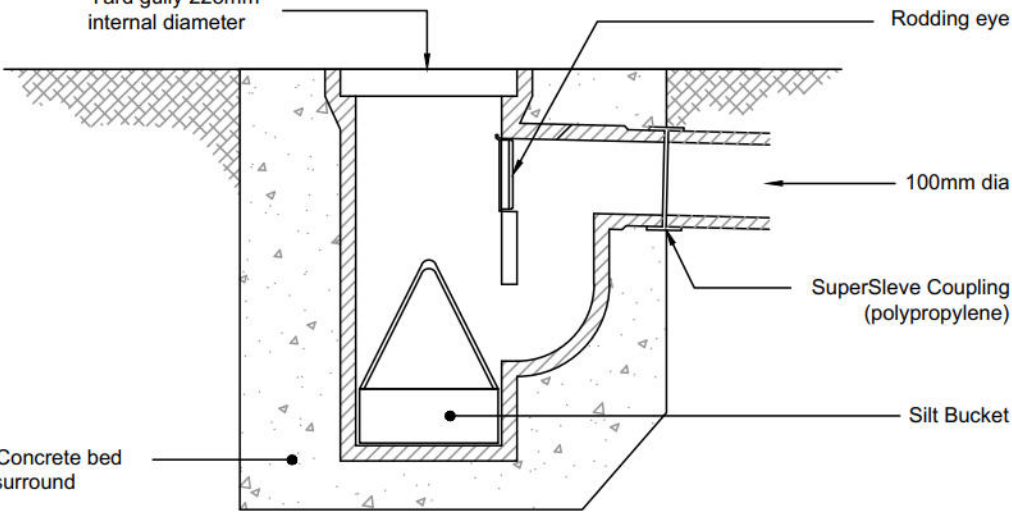
- All private drainage must comply with the current edition of DTLR Building Regulations approved document H.
- Where drainage is to be adopted it should meet with the requirements of Ofwat Code for Adoptions.
- Drainage design to be to BS EN 752-3 1996
- Any intended changes to the drainage design must be discussed with the Engineer. If changes are made the Engineer must be supplied with as-constructed information to enable drawings to be suitably updated for the Health & safety file.
- Before works commence the contractor should satisfy themselves that the details of the drainage system to be connected into are correct i.e. cover, invert levels, line, condition and type of sewer.
- Private access chambers are to be appropriate to the depths and loadings as follows:-

Depth to invert	Access size
Up to 600mm	Mini access chamber 300mmØ
Up to 1200mm	Inspection chamber 475mm Ø (PPIC)
1200 to 1500mm	600mmx450mm Brick/P.C.C units
1500 to 3000mm	P.C.C ring manhole 1050mmØ
	P.C.C. ring manhole 1200mmØ (ring diameter increased if sewer greater than 475mmØ).
- All manholes shall have a flexible joint within 150mm of the face of the structure and a "rocker pipe" which should not exceed 600mm in length.
- Pipe materials shall be:-
Vitrified clayware to BS EN 295
Cast iron to BS EN 545:2010
UPVC - BS EN 1401 PP - BS EN 1852
Structure wall - BS EN 13476
- For private sewers having 900mm or less cover beneath carriageways & hardstanding or 600mm in landscape areas then they shall have concrete surround or slab protection. Slab protection to be 100mm thick C20 concrete slab with mesh reinforcement and a bearing of 150mm each side of the trench. Concrete surround to be 150mm C20 with flexible joints.
- Trenches within 1.2m of load bearing walls should be filled with concrete at least to the underside of the foundation. Where the distance is more than 1.2m from the foundations the concrete should be taken at least up to a 45degree line from the bottom of the foundations. Alternatively, the foundations could be taken to a deeper level to avoid undermining by the drainage trench (check with the Engineer where this is required).
- Pipe bed and surround to be granular Type S unless otherwise noted.
- Drains passing through walls or foundations should have either an arched or riddled opening to give 50mm clearance around the pipe. The opening shall be masked both sides with a rigid non-perishable material, or alternatively a short length of pipe may be built in solid if it is connected within 150mm to rocker pipes (max 600mm long) with flexible joints.
- Drainage under buildings should be bedded and surrounded by at least 100mm of granular material.
- Unless otherwise stated on the drawings or in the schedules then all private drainage shall be 100mmØ.
- All road gully connections to be 150mmØ and surrounded with 150mm C20 concrete surround.
- Where schemes require soakaways they shall not be positioned closer than 5m from the nearest dwelling or structure. Where solution features can occur in the underlying strata such as chalk then this distance will need to be increased to 10m.
- New connections to existing public sewers should be carried in accordance with appropriate Section 106 (Water Industry Act) 'connection consent' and also under the supervision of the Water Authority.
- Covers shall be to B.S. EN 124:1994
Class A15 - areas where only pedestrians have access.
Class B125 - for use in car parks and pedestrian areas where occasional vehicular access is likely.
Class C250 - areas where not extending more than 500mm from kerb face into the carriageway
Class D400 - areas where cars and lorries have access including carriageways, hard shoulders.
Cover and frames to be 150mm deep except residential cul-de-sacs
- It is recommended that drainage works should be constructed from the outfall particularly where the outfall depth is relatively shallow. If it is not possible to commence works from the outfall the contractor should satisfy themselves that the invert, line, position and type of existing outfall are correct.
- Drainage works should be protected from possible damage by construction traffic loadings during the construction period. Protection may be provided by barriers, materials should not be stored over drainage works.
- Buildings up to 3 storeys shall have a rest bend at the base of the soil stack 450mm min below the invert of the lowest incoming drain. Buildings over 3 storeys must be a minimum of 750mm below the lowest incoming drain. Buildings over 5 storeys then the ground floor drainage connections should have their own connections to the external drain.
- Where piling works are undertaken the positions of existing sewers must be accurately located before piling takes place.

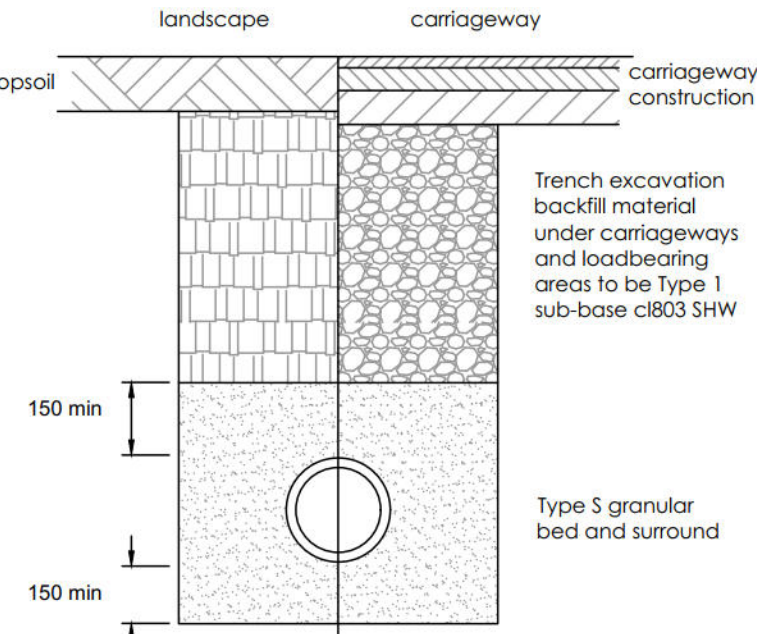
Drainage Key

Sewers

- Foul water drain (private/non adoptable)
- Surface water drain (private/non adoptable)
- Perforated Pipe (HS Powerslot or similar)
- Existing foul water drain (private/non adopted)
- Existing surface water drain (private/non adopted)
- Redundant sewer
- Yard gully (150mm - 200mmØ trapped)
- Block paving - standard (non-permeable)
- Flood exceedance routing
- Soft Landscape.



YARD GULLY



TYPE S GRANULAR SURROUND BED

To be used where cover to pipe is greater than 100mm in vehicle areas and greater than 600mm in non-trafficked areas (ie footpaths, verges, etc)

Pipe nominal size (DN)	Pipe Bedding Requirement (mm)
100	10 nominal single size
over 100-150	10 or 14mm nominal single size or 14mm to 5mm graded
over 150-300	10, 14 or 20mm nominal single-size or 14mm to 5mm graded or 20mm to 5mm graded
over 300 to 550	14, 20 or 40mm nominal single-size crushed rock or 14mm to 5mm graded or 20mm to 5mm graded
over 550	14, 20 or 40mm nominal single-size crushed rock or 14mm to 5mm graded or 20mm to 5mm graded or 40mm to 5mm graded

PIPE DIA	TRENCH WIDTH Tw
150	600
225	700
300	750
375	1050
450	1150
525	1200
600	1350
675	1450
750	1500
825	1600
900	1900
1050	2050
1200	2300

GRANULAR BEDDING AND SIDEFILL MATERIAL GRADINGS

NOTES

- All dimensions and levels are in metres unless otherwise noted
- This drawing is to be read in conjunction with the relevant Architect's/Engineer's drawings, specifications and CDM documentation
- This drawing has been produced electronically and may have been photo reduced or enlarged when copied. Work to figured dimensions only (DO NOT SCALE - EXCEPT FOR PLANNING PURPOSES). All dimensions to be checked on site. Any errors or omissions to be reported to the engineer immediately.
- This drawing contains coloured lines / information that may not be clear if reproduced in black and white.
- Digital copies of this plan can only be considered accurate if supplied directly by Infrastruct CS Ltd.

P01	RSI	RJW	Initial issue	21/07/26
REV	DRAWN	CHECK	REVISION COMMENTS	ISSUE DATE
DRAWING TITLE				SHEET NO.
Transformer Drainage				1/1
PROJECT				
Ready Power House, Molly Millar's Lane, Wokingham, RG41 2QP				
CLIENT				
E T Planning				
SCALE @ A1				
1:100				
ENGINEER: RJW				
DRAFT: RSI				
APPROVED: MBD				
PROJECT	ORIGIN	PHASE	LEVEL	TYPE
MILL	ICS	01	XX	DR
ROLE	NO.	REVISION		
C	0200	P01		



Appendix B - Hydraulic Calculations



Infrastruct CS Ltd
Consulting Civil Engineers

Existing Flows

Infrastruct CS Ltd

File: 6314-MILL-Flow.pfd

Network: Storm Network

Richard White

22/07/2026

Page 1

Design Settings

Rainfall Methodology

FSR

Return Period (years)

100

Additional Flow (%)

0

FSR Region

England and Wales

M5-60 (mm)

20.000

Ratio-R

0.400

CV

1.000

Time of Entry (mins)

5.00

Maximum Time of Concentration (mins)

30.00

Maximum Rainfall (mm/hr)

50.0

Minimum Velocity (m/s)

1.00

Connection Type

Level Soffits

Minimum Backdrop Height (m)

0.200

Preferred Cover Depth (m)

0.350

Include Intermediate Ground

✓

Enforce best practice design rules

✓

Links

Name

US Node

DS Node

Length (m)

ks (mm) / n

US IL (m)

DS IL (m)

Fall (m)

Slope (1:X)

Dia (mm)

T of C (mins)

Rain (mm/hr)

1

YG

EXMH

17.174

0.600

49.410

49.343

0.067

256.3

100

5.60

50.0

Name

Vel (m/s)

Cap (l/s)

Flow (l/s)

US Depth (m)

DS Depth (m)

Σ Area (ha)

Σ Add Inflow (l/s)

Pro Depth (mm)

Pro Velocity (m/s)

1

0.476

3.7

0.9

0.350

0.127

0.005

0.0

33

0.389

Pipeline Schedule

Link

Length (m)

Slope (1:X)

Dia (mm)

Link Type

US CL (m)

US IL (m)

US Depth (m)

DS CL (m)

DS IL (m)

DS Depth (m)

1

17.174

256.3

100

Circular_Default Sewer Type

49.860

49.410

0.350

49.570

49.343

0.127

Link

US Node

Dia (mm)

Node Type

MH Type

DS Node

Dia (mm)

Node Type

MH Type

1

YG

150

Manhole

Private

EXMH

1200

Manhole

Private

Manhole Schedule

Node

Easting (m)

Northing (m)

CL (m)

Depth (m)

Dia (mm)

Connections

Link

IL (m)

Dia (mm)

YG

896.537

130.523

49.860

0.450

150

0

1

49.410

100

EXMH

886.225

144.257

49.570

0.227

1200

1

1

49.343

100

Simulation Settings

Rainfall Methodology

FSR

Rainfall Events

Singular

FSR Region

England and Wales

M5-60 (mm)

20.000

Ratio-R

0.400

Summer CV

1.000

Winter CV

1.000

Analysis Speed

Normal

Skip Steady State

x

Drain Down Time (mins)

240

Additional Storage (m³/ha)

20.0

Starting Level (m)

Check Discharge Rate(s)

x

Check Discharge Volume

x

Storm Durations

15

30

60

120

180

240

360

480

600

720

960

1440

Return Period (years)

Climate Change (CC %)

Additional Area (A %)

Additional Flow (Q %)

1

0

0

0

2

0

0

0

Return Period (years)

Climate Change (CC %)

Additional Area (A %)

Additional Flow (Q %)

30

0

0

0

100

0

0

0

Return Period (years)

Climate Change (CC %)

Additional Area (A %)

Additional Flow (Q %)

100

40

0

0

Flow v17.0 Copyright © 1988-2026 Causeway Technologies Ltd

Node FC Online Orifice Control

Flap Valve x | Replaces Downstream Link x | Invert Level (m) 0.000 | Diameter (m) 0.020 | Discharge Coefficient 0.600

Results for 1 year Critical Storm Duration. Lowest mass balance: 100.00%

Node Event	US Node	Peak (mins)	Level (m)	Depth (m)	Inflow (l/s)	Node Vol (m³)	Flood (m³)	Status
15 minute summer	YG	10	49.445	0.035	0.9	0.0083	0.0000	OK
15 minute summer	EXMH	11	49.373	0.030	0.9	0.0000	0.0000	OK

Link Event (Upstream Depth)	US Node	Link	DS Node	Outflow (l/s)	Velocity (m/s)	Flow/Cap	Link Vol (m³)	Discharge Vol (m³)
15 minute summer	YG	1	EXMH	0.9	0.414	0.241	0.0374	0.4



Results for 2 year Critical Storm Duration. Lowest mass balance: 100.00%

Node Event	US Node	Peak (mins)	Level (m)	Depth (m)	Inflow (l/s)	Node Vol (m³)	Flood (m³)	Status
15 minute summer	YG	10	49.450	0.040	1.2	0.0095	0.0000	OK
15 minute summer	EXMH	10	49.377	0.034	1.2	0.0000	0.0000	OK
Link Event (Upstream Depth)	US Node	Link	DS Node	Outflow (l/s)	Velocity (m/s)	Flow/Cap	Link Vol (m³)	Discharge Vol (m³)
15 minute summer	YG	1	EXMH	1.2	0.444	0.310	0.0450	0.0000

Results for 30 year Critical Storm Duration. Lowest mass balance: 100.00%

Node Event	US Node	Peak (mins)	Level (m)	Depth (m)	Inflow (l/s)	Node Vol (m³)	Flood (m³)	Status
15 minute summer	YG	10	49.467	0.057	2.2	0.0137	0.0000	OK
15 minute summer	EXMH	10	49.390	0.047	2.2	0.0000	0.0000	OK
Link Event (Upstream Depth)	US Node	Link	DS Node	Outflow (l/s)	Velocity (m/s)	Flow/Cap	Link Vol (m³)	Discharge Vol (m³)
15 minute summer	YG	1	EXMH	2.2	0.526	0.579	0.0707	0.9

	Infrastruct CS Ltd	File: 6314-MILL-Flow.pfd Network: Storm Network Richard White 22/07/2026	Page 6																																													
Results for 100 year Critical Storm Duration. Lowest mass balance: 100.00% <table><tr><th>Node Event</th><th>US Node</th><th>Peak (mins)</th><th>Level (m)</th><th>Depth (m)</th><th>Inflow (l/s)</th><th>Node Vol (m³)</th><th>Flood (m³)</th><th>Status</th></tr><tr><td>15 minute summer</td><td>YG</td><td>10</td><td>49.477</td><td>0.067</td><td>2.8</td><td>0.0161</td><td>0.0000</td><td>OK</td></tr><tr><td>15 minute summer</td><td>EXMH</td><td>10</td><td>49.396</td><td>0.053</td><td>2.8</td><td>0.0000</td><td>0.0000</td><td>OK</td></tr></table> <table><tr><th>Link Event (Upstream Depth)</th><th>US Node</th><th>Link</th><th>DS Node</th><th>Outflow (l/s)</th><th>Velocity (m/s)</th><th>Flow/Cap</th><th>Link Vol (m³)</th><th>Discharge Vol (m³)</th></tr><tr><td>15 minute summer</td><td>YG</td><td>1</td><td>EXMH</td><td>2.8</td><td>0.560</td><td>0.740</td><td>0.0846</td><td>1.2</td></tr></table>				Node Event	US Node	Peak (mins)	Level (m)	Depth (m)	Inflow (l/s)	Node Vol (m³)	Flood (m³)	Status	15 minute summer	YG	10	49.477	0.067	2.8	0.0161	0.0000	OK	15 minute summer	EXMH	10	49.396	0.053	2.8	0.0000	0.0000	OK	Link Event (Upstream Depth)	US Node	Link	DS Node	Outflow (l/s)	Velocity (m/s)	Flow/Cap	Link Vol (m³)	Discharge Vol (m³)	15 minute summer	YG	1	EXMH	2.8	0.560	0.740	0.0846	1.2
Node Event	US Node	Peak (mins)	Level (m)	Depth (m)	Inflow (l/s)	Node Vol (m³)	Flood (m³)	Status																																								
15 minute summer	YG	10	49.477	0.067	2.8	0.0161	0.0000	OK																																								
15 minute summer	EXMH	10	49.396	0.053	2.8	0.0000	0.0000	OK																																								
Link Event (Upstream Depth)	US Node	Link	DS Node	Outflow (l/s)	Velocity (m/s)	Flow/Cap	Link Vol (m³)	Discharge Vol (m³)																																								
15 minute summer	YG	1	EXMH	2.8	0.560	0.740	0.0846	1.2																																								
Flow v17.0 Copyright © 1988-2026 Causeway Technologies Ltd																																																

Results for 100 year +40% CC Critical Storm Duration. Lowest mass balance: 100.00%

Node Event	US Node	Peak (mins)	Level (m)	Depth (m)	Inflow (l/s)	Node Vol (m³)	Flood (m³)	Status
15 minute summer	YG	10	49.500	0.090	4.0	0.0217	0.0000	OK
15 minute summer	EXMH	10	49.407	0.064	3.9	0.0000	0.0000	OK

Link Event (Upstream Depth)	US Node	Link	DS Node	Outflow (l/s)	Velocity (m/s)	Flow/Cap	Link Vol (m³)	Discharge Vol (m³)
15 minute summer	YG	1	EXMH	3.9	0.603	1.043	0.1093	1.7



Proposed Flows

Infrastruct CS Ltd

File: 6314-MILL-Flow.pfd

Network: Storm Network

Richard White

22/07/2026

Page 1

Design Settings

Rainfall Methodology

FSR

Return Period (years)

100

Additional Flow (%)

0

FSR Region

England and Wales

M5-60 (mm)

20.000

Ratio-R

0.400

CV

1.000

Time of Entry (mins)

5.00

Maximum Time of Concentration (mins)

30.00

Maximum Rainfall (mm/hr)

50.0

Minimum Velocity (m/s)

1.00

Connection Type

Level Soffits

Minimum Backdrop Height (m)

0.200

Preferred Cover Depth (m)

0.350

Include Intermediate Ground

✓

Enforce best practice design rules

✓

Links

Name

US Node

DS Node

Length (m)

ks (mm) / n

US IL (m)

DS IL (m)

Fall (m)

Slope (1:X)

Dia (mm)

T of C (mins)

Rain (mm/hr)

1

YG

EXMH

17.174

0.600

49.410

49.343

0.067

256.3

100

5.60

50.0

Name

Vel (m/s)

Cap (l/s)

Flow (l/s)

US Depth (m)

DS Depth (m)

Σ Area (ha)

Σ Add Inflow (l/s)

Pro Depth (mm)

Pro Velocity (m/s)

1

0.476

3.7

0.5

0.350

0.127

0.003

0.0

26

0.340

Pipeline Schedule

Link

Length (m)

Slope (1:X)

Dia (mm)

Link Type

US CL (m)

US IL (m)

US Depth (m)

DS CL (m)

DS IL (m)

DS Depth (m)

1

17.174

256.3

100

Circular_Default Sewer Type

49.860

49.410

0.350

49.570

49.343

0.127

Link

US Node

Dia (mm)

Node Type

MH Type

DS Node

Dia (mm)

Node Type

MH Type

1

YG

150

Manhole

Private

EXMH

1200

Manhole

Private

Manhole Schedule

Node

Easting (m)

Northing (m)

CL (m)

Depth (m)

Dia (mm)

Connections

Link

IL (m)

Dia (mm)

YG

896.537

130.523

49.860

0.450

150

0

1

49.410

100

EXMH

886.225

144.257

49.570

0.227

1200

1

1

49.343

100

Simulation Settings

Rainfall Methodology

FSR

Rainfall Events

Singular

FSR Region

England and Wales

M5-60 (mm)

20.000

Ratio-R

0.400

Summer CV

1.000

Winter CV

1.000

Analysis Speed

Normal

Skip Steady State

x

Drain Down Time (mins)

240

Additional Storage (m³/ha)

20.0

Starting Level (m)

Check Discharge Rate(s)

x

Check Discharge Volume

x

Storm Durations

15

30

60

120

180

240

360

480

600

720

960

1440

Return Period (years)

Climate Change (CC %)

Additional Area (A %)

Additional Flow (Q %)

1

0

0

0

2

0

0

0

Return Period (years)

Climate Change (CC %)

Additional Area (A %)

Additional Flow (Q %)

30

0

0

0

100

0

0

0

Return Period (years)

Climate Change (CC %)

Additional Area (A %)

Additional Flow (Q %)

100

40

0

0

Flow v17.0 Copyright © 1988-2026 Causeway Technologies Ltd



Node FC Online Orifice Control

Flap Valve x | Replaces Downstream Link x | Invert Level (m) 0.000 | Diameter (m) 0.020 | Discharge Coefficient 0.600

Results for 1 year Critical Storm Duration. Lowest mass balance: 100.00%

Node Event	US Node	Peak (mins)	Level (m)	Depth (m)	Inflow (l/s)	Node Vol (m³)	Flood (m³)	Status
15 minute summer	YG	11	49.436	0.026	0.5	0.0039	0.0000	OK
15 minute summer	EXMH	11	49.365	0.022	0.5	0.0000	0.0000	OK

Link Event (Upstream Depth)	US Node	Link	DS Node	Outflow (l/s)	Velocity (m/s)	Flow/Cap	Link Vol (m³)	Discharge Vol (m³)
15 minute summer	YG	1	EXMH	0.5	0.351	0.134	0.0245	0.2

Results for 2 year Critical Storm Duration. Lowest mass balance: 100.00%

Node Event	US Node	Peak (mins)	Level (m)	Depth (m)	Inflow (l/s)	Node Vol (m³)	Flood (m³)	Status
15 minute summer	YG	11	49.440	0.030	0.7	0.0046	0.0000	OK
15 minute summer	EXMH	11	49.369	0.026	0.7	0.0000	0.0000	OK

Link Event (Upstream Depth)	US Node	Link	DS Node	Outflow (l/s)	Velocity (m/s)	Flow/Cap	Link Vol (m³)	Discharge Vol (m³)
15 minute summer	YG	1	EXMH	0.7	0.386	0.187	0.0312	0.3

Results for 30 year Critical Storm Duration. Lowest mass balance: 100.00%

Node Event	US Node	Peak (mins)	Level (m)	Depth (m)	Inflow (l/s)	Node Vol (m³)	Flood (m³)	Status
15 minute summer	YG	10	49.452	0.042	1.3	0.0064	0.0000	OK
15 minute summer	EXMH	10	49.379	0.036	1.3	0.0000	0.0000	OK

Link Event (Upstream Depth)	US Node	Link	DS Node	Outflow (l/s)	Velocity (m/s)	Flow/Cap	Link Vol (m³)	Discharge Vol (m³)
15 minute summer	YG	1	EXMH	1.3	0.459	0.348	0.0488	0.6

Results for 100 year Critical Storm Duration. Lowest mass balance: 100.00%

Node Event	US Node	Peak (mins)	Level (m)	Depth (m)	Inflow (l/s)	Node Vol (m³)	Flood (m³)	Status
15 minute summer	YG	10	49.459	0.049	1.7	0.0074	0.0000	OK
15 minute summer	EXMH	10	49.384	0.041	1.7	0.0000	0.0000	OK

Link Event (Upstream Depth)	US Node	Link	DS Node	Outflow (l/s)	Velocity (m/s)	Flow/Cap	Link Vol (m³)	Discharge Vol (m³)
15 minute summer	YG	1	EXMH	1.7	0.490	0.446	0.0584	0.7

Results for 100 year +40% CC Critical Storm Duration. Lowest mass balance: 100.00%

Node Event	US Node	Peak (mins)	Level (m)	Depth (m)	Inflow (l/s)	Node Vol (m³)	Flood (m³)	Status
15 minute summer	YG	10	49.470	0.060	2.4	0.0091	0.0000	OK
15 minute summer	EXMH	10	49.392	0.049	2.4	0.0000	0.0000	OK

Link Event (Upstream Depth)	US Node	Link	DS Node	Outflow (l/s)	Velocity (m/s)	Flow/Cap	Link Vol (m³)	Discharge Vol (m³)
15 minute summer	YG	1	EXMH	2.4	0.538	0.633	0.0755	1.1