

Development 10 Year Plot Delivery Targets											
Phase	Units	Year 1	Year 2	Year 3	Year 4	Year 5	Year 6	Year 7	Year 8	Year 9	Year 10
1	178	41	68	69							
2	102		25	65	12						
3	24		24								
4	69		25	44							
5	136			25	65	46					
6	142				25	65	52				
7	105			15	65	25					
8	34				25	9					
9	66					25	41				
10	179						25	65	65	14	
11	163						25	65	65	8	
12a	78							25	53		
12b	16							16			
13a	43									25	18
13b	13									13	
14	19										19
15	119								25	65	29
Rolling Total		41	183	401	593	763	906	1077	1285	1410	1476

Plot Delivery Target Notes:

- Delivery targets based on Finchampstead Year on Year Plot Delivery Target schedule provided by Cooper Ballie and dated 17/10/2019.
- Thames Water have confirmed that the existing foul water network has capacity for flows from up to 200 plots. Based on the above targets, completion of the upgrade works to the existing Thames Water foul sewer network will be required from the start of Year 3 to accommodate the flows from the rest of the development.
-

Foul Discharge Rates

Foul flow rates have been calculated in accordance with the requirements set out in Sewers for Adoption 7th Edition. The rates stated on this drawing are indicative only, based on masterplan information and assumed minimum pumped rates.

Pumped rates to be confirmed by MSE pump designer.

Peak design flow rates based on the following:

- flows from dwellings based on 4000 litres per dwelling per day
- flows from school based on 0.6 l/s/m²
- total flows from employment area based on 0.6 l/s/m² for domestic flows and 0.7 l/s/m² for trade effluent flows

Maximum discharge from site to be 39.78 l/s in accordance with the Thames Water Sewer Impact Study ref: X4503-1255-SMG2102 V2.0, dated September 2017.

Indicative temporary and permanent discharge rates for the development are summarised in Table 1 and Table 2.

* flows from the school and employment areas are indicative only and subject to discussion with Thames Water to confirm the flow calculation method.

Table 1 - Anticipated Temporary Pumped Foul Flow Rates	
Area	Flow Rate
Temporary Pump Station 1 - Phase 1	6.1 l/s (pumped) TBC
Temporary Pump Station 2 - Phases 1, 2 and 3	7.1 l/s (pumped) TBC

Table 2 - Anticipated Final Development Foul Flow Rates	
Area	Flow Rate
Permanent Adoptable Pump Station 2 - Phases 1-9	19.85 l/s (pumped) TBC
Eastern Pump Station - Phases 10-15 (including 13a)	14.40 l/s (pumped) TBC
School & Neighbourhood Centre	1.40 l/s (0.6 l/s/m²)
Gravely Sewers - 10 plots from Phase 10, 10 plots from Phase 11, 10 plots from Phase 12a, Phase 13a & Employment Area	6.42 l/s (gravity) TBC
	Σ = 40.67 l/s TBC

New manhole on ex. Thames Water 3000 foul sewer
CL 56.45
IL 53.35 (Interpolated) TBC

Connection point based on TWU 'Foul Water Impact Assessment' ref. SMG 2102 which advised that capacity exists for up to 200 units in this FW sewer without the need for any sewer improvement works. The existing manhole given as the point of connection is SU77645601, which is upstream of the proposed new manhole connection.

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- Key:**
- Foul Water Pump Station with 15m Cordons Sanitary
 - Masterplan Site Boundary
 - Indicative Proposed Permanent Foul Rising Main
 - Indicative Proposed Temporary Rising Main
 - Indicative Phase Boundaries
 - Indicative Proposed Foul Gravity Sewer
 - Existing Thames Water Foul Sewer
 - Ancient Woodland
 - Indicative Parcel Outlines to Proposed Gravity Sewer

- General Notes**
- All foul sewer and pump station discharge rates are indicative only and to be confirmed by Thames Water.
 - Topography based on survey provided by Brunel Surveys Ltd January 2018 (reference: 18215-Hogwood Farm 2D 09.01.18).
 - OS Data: © Crown copyright and database rights 2016 Ordnance Survey 0100031673
 - Concept Masterplan provided by Cooper Ballie (reference: CB_12_103_A00_Rev_L, Date: April 2018)
 - Parcel phasing and number of plots based on Cooper Ballie Phasing Parameter Plan ref: CB_12_103_S73_006 and Hogwood Overall Mix Schedule ref: CB_12_103_103, dated 20.08.2018.
 - School, Neighbourhood Centre and Employment areas based on Cooper Ballie Land Use Parameter Plan ref: CB_12_103_S73_001 Rev J, dated April 2018.

Rev.	Date	Amendment	By	Chk.

Preliminary



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Client: Legal and General Homes
Job: Hogwood Farm Finchampstead
Title: Development Foul Drainage Phasing Plan

Scale: 1:1000
Drawn By: KM
Eng. Check: HAS
Tech. Check: HAS
Drawing No.: C85672-SK-220
Rev.: -



Phase 1
178 Plots
Foul outfall will be staged as follows:
1. Discharge via gravity to Phase 1 Temporary Foul Pump Station during initial construction phasing.
2. Foul flows to discharge via gravity to Phase 1/2/3 Temporary Pump Station once operational.
3. Foul flows to ultimately discharge to Adopted Permanent Pump Station 1 once operational.
Peak Flow Rate = 8.24 l/s

Phase 9
66 Plots
Discharge via Permanent Private Pump to gravity sewer within main infrastructure.
Adopted Permanent Foul Pump Station 1.
Peak Flow Rate = 3.06 l/s
Pumped flows TBC

Phase 8
34 Plots
Discharge via gravity to Adopted Permanent Foul Pump Station 1
Peak Flow Rate = 1.57 l/s

Phase 1 Private Temporary Pump Station
Compound size 5m x 10m
Total plots connected = 178
Emergency storage required = 28.48m³
Total peak flow rate from catchment = 8.24 l/s
Pumped flow = 6.1 l/s
Temporary pump station to be based on a semi hybrid SFA station previously used in this area where building control allowed the use of SFA storage guidelines of 160dwelling subject to the equipment and telemetry being of a good standard rather than the 150 litres per person as building regulations, thus reducing storage.
Pumps to be selected on the basis that they can be moved at a later date to Phase 1/2/3 Temporary Pump Station, TBC.
Refer to PDAS Phase 1 Temporary Pump Station design report ref: PR10785 and drawings PR10785 - 001 to 007 for details.
Temporary pump station to be decommissioned once development occupation progresses past Phase 1

Phase 3
24 Plots
Discharge to Phase 1/2/3 Temporary Foul Pump Station
Peak Flow Rate = 1.11 l/s

Phase 2
102 Plots
Discharge to Phase 1/2/3 Temporary Foul Pump Station
Peak Flow Rate = 4.72 l/s

Phase 4
69 Plots
Discharge via gravity to Adopted Permanent Foul Pump Station 1
Peak Flow Rate = 3.19 l/s

Phase 7
105 Plots
Discharge via gravity to Adopted Permanent Foul Pump Station 1
Peak Flow Rate = 4.86 l/s

Phase 1/2/3 Private Temporary Pump Station
Compound size 5m x 10m
Total plots connected = 304
Emergency storage required = 48.64m³
Total peak flow rate from catchment = 14.07 l/s
Pumped flow = 7.1 l/s
Temporary pump station to be based on a semi hybrid SFA station previously used in this area where building control allowed the use of SFA storage guidelines of 160dwelling subject to the equipment and telemetry being of a good standard rather than the 150 litres per person as building regulations, thus reducing storage.
Pumps from Phase 1 Temporary Pump Station to potentially be re-used for this pump station, TBC.
Temporary pump station to be decommissioned once development construction moves past Phase 3.

Phase 6
142 Plots
Discharge via gravity to Adopted Permanent Foul Pump Station 1
Peak Flow Rate = 6.57 l/s

Phase 5
136 Plots
Discharge via gravity to Adopted Permanent Foul Pump Station 1
Peak Flow Rate = 6.30 l/s

Adopted Permanent Foul Pump Station 1
Size and Location TBC
Preliminary compound size 10m x 12m with 15m offset to nearest building
Total plots connected = 856
Emergency storage required = 136.97m³
Total peak flow rate from catchment = 39.63 l/s
Indicative pumped flow (to achieve rising main self-cleansing) = 19.85 l/s
Note: Pumped flows should be at least the maximum of:
a. half the peak design flow rate
b. the flow rate required to achieve a minimum flow velocity in the rising main of 0.75 m/s
Will require construction when development moves past Phase 3 or if the order of construction differs from the Phases agreed at present.

Adopted Permanent Foul Pump Station 2
Size and Location TBC
Preliminary compound size 10m x 12m with 15m offset to nearest building
Total plots connected = 571
Emergency storage required = 91.36m³
Total peak flow rate from catchment = 29.23 l/s
Indicative pumped flow (half peak design flow rate) = 14.40 l/s TBC
Note: Pumped flows should be at least the maximum of:
a. half the peak design flow rate
b. the flow rate required to achieve a minimum flow velocity in the rising main of 0.75 m/s
Will require construction when development moves past Phase 9, or if the order of construction differs from the Phases agreed at present.
Pumps may need to be replaced at certain stages during the development of the eastern phases due to lower flows in the early stages of development.
There may be a requirement for temporary pump stations serving the eastern phases if the order of construction differs to the 10 year plot delivery targets.

Employment Area
2,337m²
Discharge to gravity sewers
Domestic Peak Flow Rate = 1.40 l/s (0.6 l/s/m²)
Trade Effluent Flow Rate = 1.64 (0.7 l/s/m²)
Total Peak Flow Rate = 3.04 l/s
Employment area rates to be confirmed by Thames Water

Rising mains from Western and Eastern Adoptable Foul Pump Stations to discharge to foul manhole chamber prior to outfalling via gravity to the proposed connection point in Park Lane.
Refer to drawing C85672-SK-217 for Western Phases Temporary & Permanent Rising Main Strategy

Phase 10
173 Plots
Total Peak Flow Rate = 8.29 l/s
10 Plots to discharge to gravity sewer
Peak Flow Rate = 0.46 l/s
153 Plots to discharge to Adopted Permanent Foul Pump Station 2
Peak Flow Rate = 7.83 l/s

Phase 11
163 Plots
Total Peak Flow Rate = 7.55 l/s
10 Plots to discharge to gravity sewer
Peak Flow Rate = 0.46 l/s
153 Plots to discharge to Adopted Permanent Foul Pump Station 2
Peak Flow Rate = 7.09 l/s

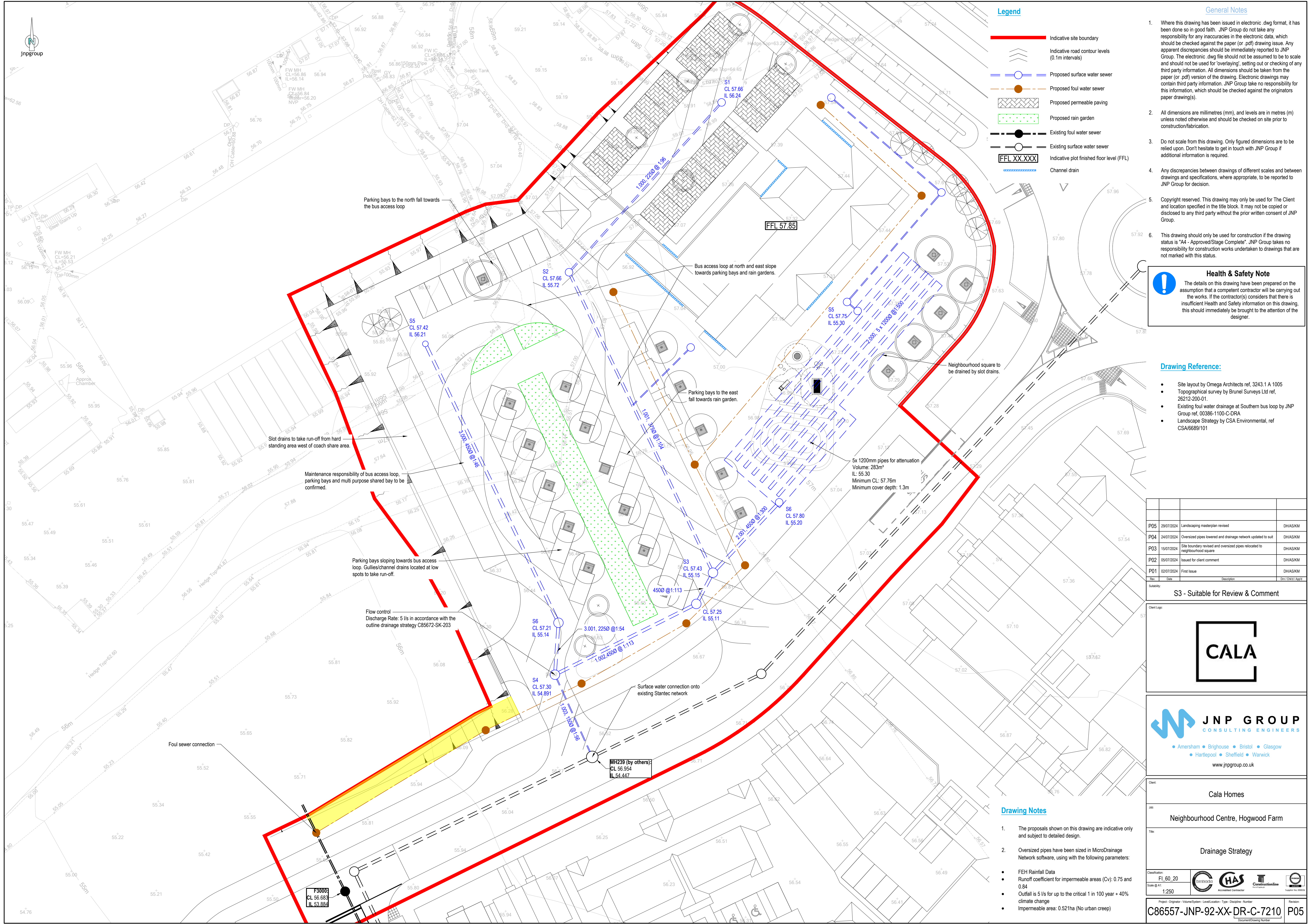
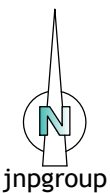
Phase 12a
78 Plots
Total Peak Flow Rate = 3.61 l/s
10 Plots to discharge to gravity sewer
Peak Flow Rate = 0.46 l/s
68 Plots to discharge to Adopted Permanent Foul Pump Station 2
Peak Flow Rate = 3.15 l/s

Phase 12b
16 Plots
Discharge to Adopted Permanent Foul Pump Station 2
Peak Flow Rate = 0.74 l/s

Phase 14
19 Plots
Discharge to Adopted Permanent Foul Pump Station 2
Peak Flow Rate = 0.88 l/s

Phase 15
119 Plots
Discharge to Adopted Permanent Foul Pump Station 2
Peak Flow Rate = 5.51 l/s

APPENDIX C: DRAINAGE STRATEGY & SURFACE WATER HYDRAULIC CALCULATIONS



Legend

- Indicative site boundary
- Indicative road contour levels (0.1m intervals)
- Proposed surface water sewer
- Proposed foul water sewer
- Proposed permeable paving
- Proposed rain garden
- Existing foul water sewer
- Existing surface water sewer
- Indicative plot finished floor level (FFL)
- Channel drain

General Notes

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Drawing Reference:

- Site layout by Omega Architects ref. 3243.1 A 1005
- Topographical survey by Brunel Surveys Ltd ref. 26212-200-01.
- Existing foul water drainage at Southern bus loop by JNP Group ref. 00386-1100-C-DRA
- Landscape Strategy by CSA Environmental, ref. CSA6689/101

Rev	Date	Description	Drawn / Checked / Approved
P05	29/07/2024	Landscaping masterplan revised	DH/ASKM
P04	24/07/2024	Oversized pipes lowered and drainage network updated to suit	DH/ASKM
P03	15/07/2024	Site boundary revised and oversized pipes relocated to neighbourhood square	DH/ASKM
P02	05/07/2024	Issued for client comment	DH/ASKM
P01	02/07/2024	First issue	DH/ASKM

S3 - Suitable for Review & Comment

Client Logo:

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Client: **Cala Homes**

Job: **Neighbourhood Centre, Hogwood Farm**

Title: **Drainage Strategy**

Classification: **FL_60_20**
Scale @ A1: **1:250**

Project - Originator - Volume/System - Level/Location - Type - Discipline - Number
C86557-JNP-92-XX-DR-C-7210

Reason: **P05**

Document/Drawing Number

Drawing Notes

- The proposals shown on this drawing are indicative only and subject to detailed design.
- Oversized pipes have been sized in MicroDrainage Network software, using with the following parameters:
 - FEH Rainfall Data
 - Runoff coefficient for impermeable areas (Cv): 0.75 and 0.84
 - Outfall is 5 l/s for up to the critical 1 in 100 year + 40% climate change
 - Impermeable area: 0.521ha (No urban creep)

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Date 15/07/2024 File 2024-07-011 SW	Designed by DH Checked by AS	
XP Solutions	Network 2020.1.3	

STORM SEWER DESIGN by the Modified Rational Method

Design Criteria for Storm

Pipe Sizes STANDARD Manhole Sizes STANDARD

FEH Rainfall Model

Return Period (years)	100
FEH Rainfall Version	2013
Site Location GB 477277 164312 SU 77277 64312	
Data Type	Point
Maximum Rainfall (mm/hr)	50
Maximum Time of Concentration (mins)	30
Foul Sewage (l/s/ha)	0.000
Volumetric Runoff Coeff.	0.750
PIMP (%)	100
Add Flow / Climate Change (%)	40
Minimum Backdrop Height (m)	0.200
Maximum Backdrop Height (m)	1.500
Min Design Depth for Optimisation (m)	1.200
Min Vel for Auto Design only (m/s)	1.00
Min Slope for Optimisation (1:X)	500

Designed with Level Soffits

Network Design Table for Storm

« - Indicates pipe capacity < flow

PN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)	T.E. (mins)	Base Flow (l/s)	k (mm)	HYD SECT	DIA (mm)	Section Type	Auto Design
1.000	35.533	0.370	96.0	0.058	5.00	0.0	0.600	o	225	Pipe/Conduit	
1.001	51.859	0.412	125.9	0.157	0.00	0.0	0.600	o	375	Pipe/Conduit	
2.000	50.000	0.100	500.0	0.119	5.00	0.0	0.600	5o	-6	Pipe/Conduit	
2.001	15.673	0.337	46.5	0.043	0.00	0.0	0.600	o	450	Pipe/Conduit	
1.002	29.120	0.342	85.1	0.010	0.00	0.0	0.600	o	450	Pipe/Conduit	

Network Results Table

PN	Rain (mm/hr)	T.C. (mins)	US/IL (m)	Σ I.Area (ha)	Σ Base Flow (l/s)	Foul (l/s)	Add Flow (l/s)	Vel (m/s)	Cap (l/s)	Flow (l/s)
1.000	50.00	5.44	56.240	0.058	0.0	0.0	3.1	1.33	53.1	11.0
1.001	50.00	5.98	55.720	0.215	0.0	0.0	11.6	1.61	178.2	40.8
2.000	50.00	5.48	55.660	0.119	0.0	0.0	6.4	1.72	8254.0	22.6
2.001	50.00	5.57	55.560	0.162	0.0	0.0	8.8	2.99	475.1	30.7
1.002	50.00	6.20	55.223	0.387	0.0	0.0	21.0	2.20	350.6	73.4

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Network Design Table for Storm

PN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)	T.E. (mins)	Base Flow (l/s)	k (mm)	HYD SECT	DIA (mm)	Section Type	Auto Design
3.000	49.000	0.623	78.7	0.125	5.00	0.0	0.600	o	450	Pipe/Conduit	
3.001	13.500	0.251	53.8	0.007	0.00	0.0	0.600	o	450	Pipe/Conduit	
1.003	12.195	0.219	55.7	0.002	0.00	0.0	0.600	o	150	Pipe/Conduit	

Network Results Table

PN	Rain (mm/hr)	T.C. (mins)	US/IL (m)	Σ I.Area (ha)	Σ Base Flow (l/s)	Foul (l/s)	Add Flow (l/s)	Vel (m/s)	Cap (l/s)	Flow (l/s)
3.000	50.00	5.36	56.210	0.125	0.0	0.0	6.8	2.29	364.9	23.7
3.001	50.00	5.44	55.142	0.132	0.0	0.0	7.1	2.78	441.7	25.0
1.003	50.00	6.35	54.891	0.521	0.0	0.0	28.2	1.35	23.9«	98.8

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Area Summary for Storm

Pipe Number	PIMP Type	PIMP Name	PIMP (%)	Gross Area (ha)	Imp. Area (ha)	Pipe Total (ha)
1.000	-	-	100	0.058	0.058	0.058
1.001	-	-	100	0.157	0.157	0.157
2.000	-	-	100	0.119	0.119	0.119
2.001	-	-	100	0.043	0.043	0.043
1.002	-	-	100	0.010	0.010	0.010
3.000	-	-	100	0.125	0.125	0.125
3.001	-	-	100	0.007	0.007	0.007
1.003	-	-	100	0.002	0.002	0.002
				Total	Total	Total
				0.521	0.521	0.521

Free Flowing Outfall Details for Storm

Outfall Pipe Number	Outfall Name	C. Level (m)	I. Level (m)	Min I. Level (m)	D,L (mm)	W (mm)
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1.003		56.954	54.672	0.000	0	0
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Simulation Criteria for Storm

Volumetric Runoff Coeff	0.750	Additional Flow - % of Total Flow	0.000
Areal Reduction Factor	1.000	MADD Factor * 10m ³ /ha Storage	2.000
Hot Start (mins)	0	Inlet Coefficient	0.800
Hot Start Level (mm)	0	Flow per Person per Day (l/per/day)	0.000
Manhole Headloss Coeff (Global)	0.500	Run Time (mins)	60
Foul Sewage per hectare (l/s)	0.000	Output Interval (mins)	1
Number of Input Hydrographs 0			
Number of Storage Structures 0			
Number of Online Controls 1			
Number of Time/Area Diagrams 0			
Number of Offline Controls 0			
Number of Real Time Controls 0			

Synthetic Rainfall Details

Rainfall Model	FEH
Return Period (years)	100
FEH Rainfall Version	2013
Site Location	GB 477277 164312 SU 77277 64312
Data Type	Point
Summer Storms	Yes
Winter Storms	No
Cv (Summer)	0.750
Cv (Winter)	0.840
Storm Duration (mins)	30

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Online Controls for Storm

Hydro-Brake® Optimum Manhole: 4, DS/PN: 1.003, Volume (m³): 14.1

Unit Reference	MD-SHE-0089-5000-2240-5000
Design Head (m)	2.240
Design Flow (l/s)	5.0
Flush-Flo™	Calculated
Objective	Minimise upstream storage
Application	Surface
Sump Available	Yes
Diameter (mm)	89
Invert Level (m)	54.891
Minimum Outlet Pipe Diameter (mm)	150
Suggested Manhole Diameter (mm)	1200

Control Points	Head (m)	Flow (l/s)	Control Points	Head (m)	Flow (l/s)
Design Point (Calculated)	2.240	5.0	Kick-Flo®	0.794	3.1
Flush-Flo™	0.387	3.9	Mean Flow over Head Range	-	3.8

The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated

Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)
0.100	2.8	1.200	3.7	3.000	5.7	7.000	8.5
0.200	3.6	1.400	4.0	3.500	6.1	7.500	8.8
0.300	3.8	1.600	4.3	4.000	6.5	8.000	9.1
0.400	3.9	1.800	4.5	4.500	6.9	8.500	9.4
0.500	3.8	2.000	4.7	5.000	7.3	9.000	9.6
0.600	3.7	2.200	4.9	5.500	7.6	9.500	9.9
0.800	3.1	2.400	5.2	6.000	7.9		
1.000	3.4	2.600	5.3	6.500	8.2		

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2 year Return Period Summary of Critical Results by Maximum Level (Rank 1)
for Storm

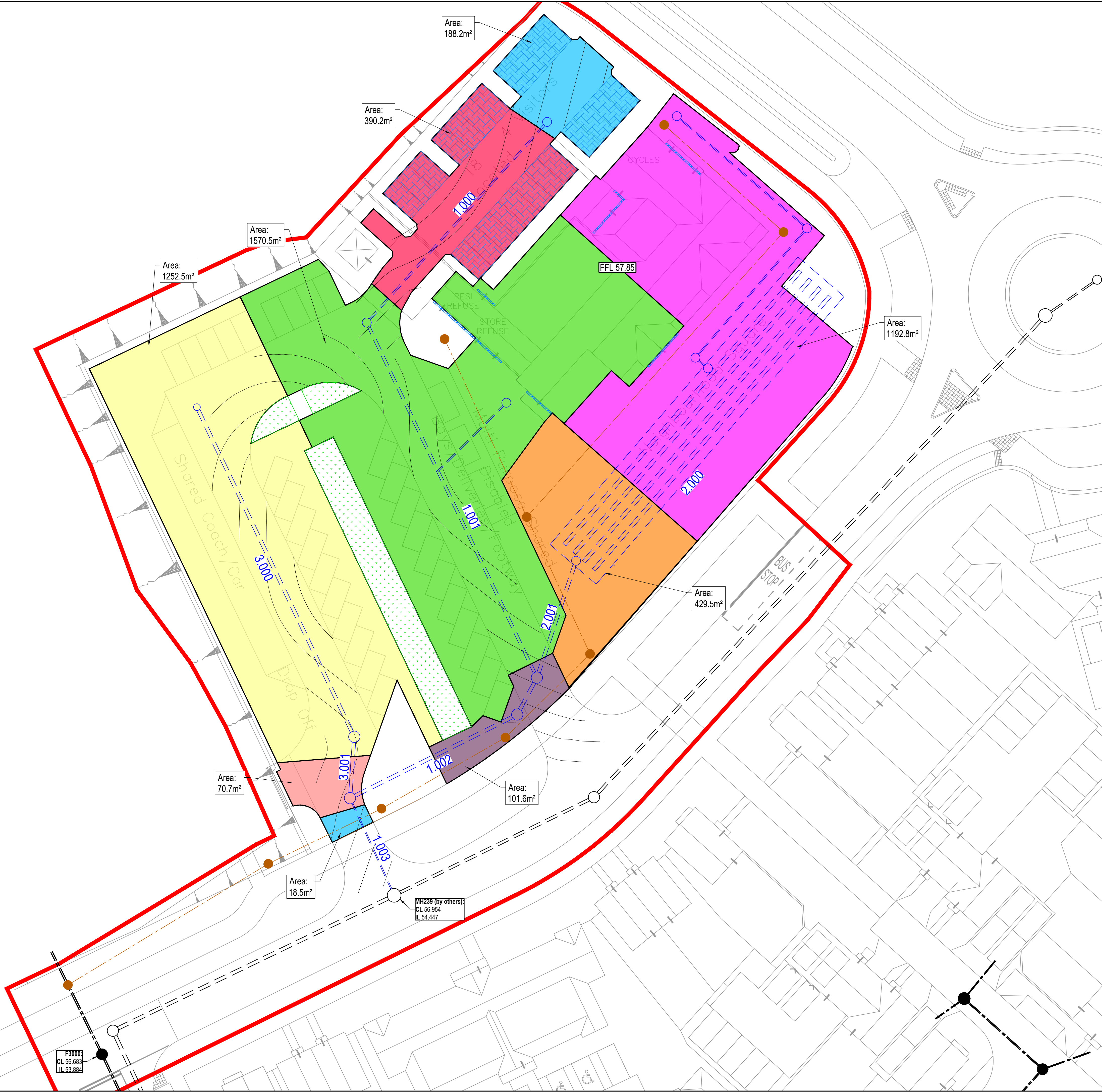
PN	US/MH Name	Surcharged Depth (m)	Flooded Volume (m ³)	Flow / Cap.	Overflow (l/s)	Half Drain Time (mins)	Pipe Flow (l/s)	Status	Level Exceeded
1.000	1	-0.159	0.000	0.19			9.3	OK	
1.001	2	-0.123	0.000	0.05			8.9	OK	
2.000	5	-0.889	0.000	0.00			4.2	OK	
2.001	6	-0.039	0.000	0.01			4.4	OK	
1.002	3	0.298	0.000	0.02			7.1	SURCHARGED	
3.000	7	-0.378	0.000	0.06			19.9	OK	
3.001	8	0.378	0.000	0.02			4.8	SURCHARGED	
1.003	4	0.929	0.000	0.18			3.9	SURCHARGED	

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Date 15/07/2024 File 2024-07-011 SW	Designed by DH Checked by AS	
XP Solutions	Network 2020.1.3	

100 year Return Period Summary of Critical Results by Maximum Level (Rank 1) for Storm

PN	US/MH Name	Surcharged Depth (m)	Flooded Volume (m ³)	Flow / Cap.	Overflow (l/s)	Half Drain Time (mins)	Pipe Flow (l/s)	Status	Level Exceeded
1.000	1	0.706	0.000	0.13			6.7	SURCHARGED	
1.001	2	1.074	0.000	0.15			24.3	SURCHARGED	
2.000	5	0.308	0.000	0.00			12.7	SURCHARGED	
2.001	6	1.158	0.000	0.01			4.2	SURCHARGED	
1.002	3	1.495	0.000	0.03			7.5	FLOOD RISK	
3.000	7	0.506	0.000	0.04			14.5	FLOOD RISK	
3.001	8	1.574	0.000	0.05			13.4	FLOOD RISK	
1.003	4	2.125	0.000	0.23			5.0	FLOOD RISK	

APPENDIX D: SURFACE WATER DRAINAGE AREA PLAN



General Notes

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Health & Safety Note

The details on this drawing have been prepared on the assumption that a competent contractor will be carrying out the works. If the contractor(s) considers that there is insufficient Health and Safety information on this drawing, this should immediately be brought to the attention of the designer.

Legend

- Indicative site boundary
- Indicative road contour levels (0.1m intervals)
- Proposed surface water sewer
- Proposed foul water sewer
- Proposed permeable paving
- Proposed rain garden
- 3m Easement
- Existing foul water sewer
- Existing surface water sewer
- FFL XX.XXX Indicative plot finished floor level (FFL)

P02	15/07/2024	Drainage strategy revised and catchment areas updated to suit	DH/AS/KM
P01	05/07/2024	First Issue	JL/ND/HAS
Rev	Date	Description	Drawn / Check / App'd

Subsidiary: S3 - Suitable for Review & Comment

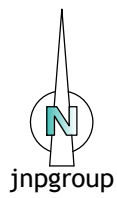


Client: CALA Homes
Job: Hogwood Farm Neighbourhood Centre
Title: Pipe Catchment Layout

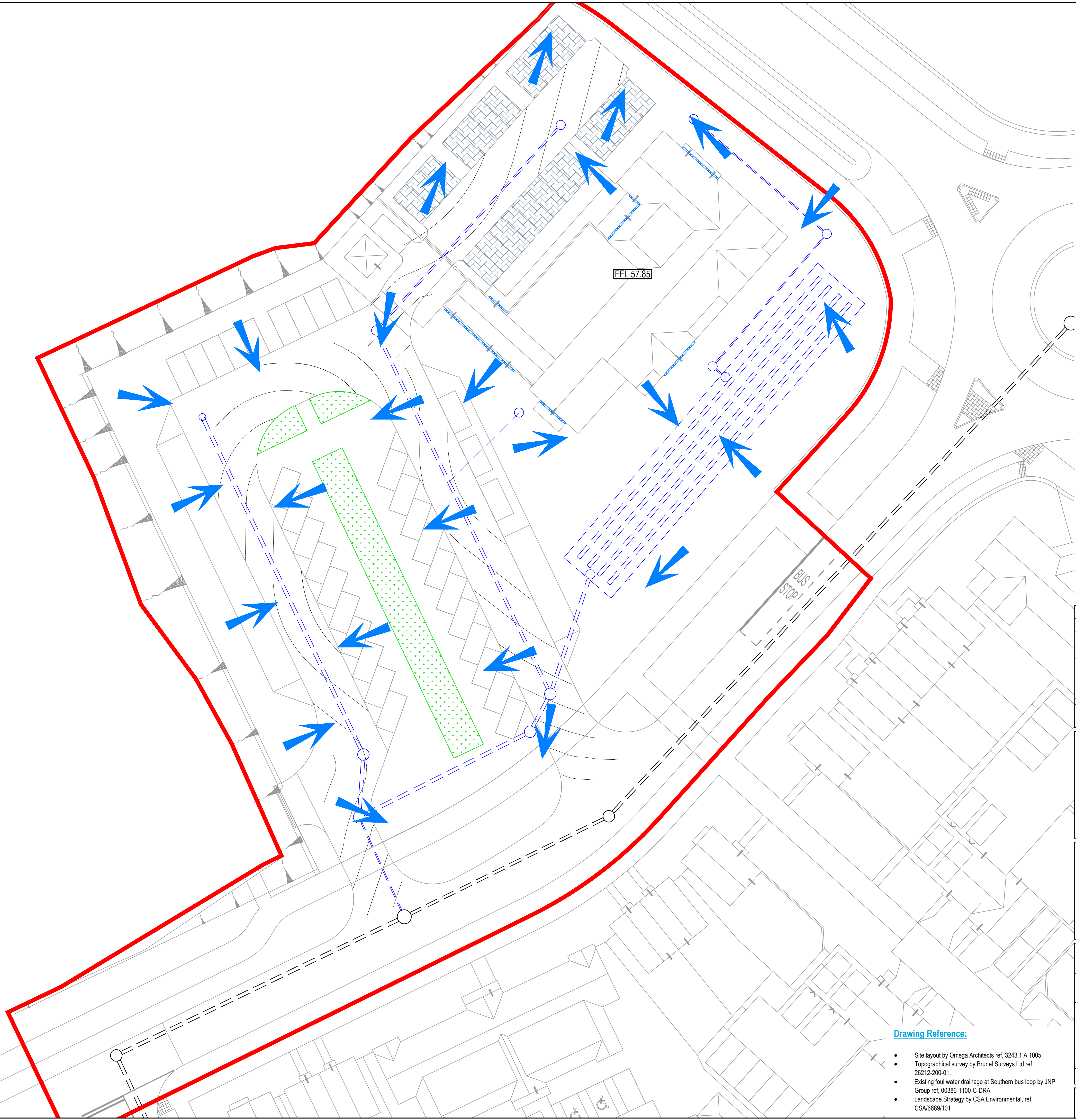
Classification: FL_60_20	Construction	HAS	Constructionline	Supplier to 00000
Scale @ A1: 1:250	Accredited Contractor			

Project - Originator - Volume/System - Level/Location - Type - Discipline - Number
C86557-JNP-XX-XX-DR-C-7212 P02

APPENDIX E: FLOOD EXCEEDANCE FLOW PATHS



- Indicative site boundary
- Indicative road contour levels (0.1m intervals)
- Proposed surface water sewer
- Proposed foul water sewer
- Proposed permeable paving
- Proposed rain garden
- Existing foul water sewer
- Existing surface water sewer
- Indicative plot finished floor level (FFL)
- Indicative overland route of exceedance flood flows



General Notes

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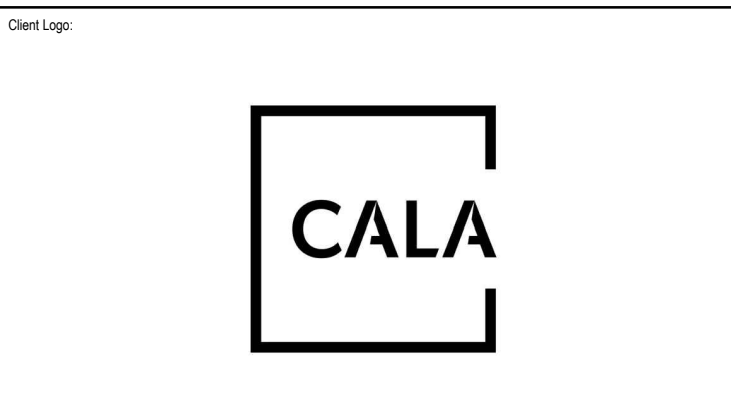
Health & Safety Note

The details on this drawing have been prepared on the assumption that a competent contractor will be carrying out the works. If the contractor(s) considers that there is insufficient Health and Safety information on this drawing, this should immediately be brought to the attention of the designer.

P02	15/07/2024	Drainage strategy revised	DH/ASKM
P01	05/07/2024	First Issue	AS/DHAS
Rev	Date	Description	Drawn / Checked / App'd

Stability:

S3 - Suitable for Review & Comment



Client: Cala Homes

Job: Neighbourhood Centre, Hogwood Farm

Title: Exceedance Flow Layout

Drawing Reference:

- Site layout by Omega Architects ref, 3243.1 A 1005
- Topographical survey by Brunel Surveys Ltd ref, 26212-200-01.
- Existing foul water drainage at Southern bus loop by JNP Group ref, 00386-1100-C-DRA
- Landscape Strategy by CSA Environmental, ref CSA/6689/101

Classification:	FL_60_20				
Scale @ A1:	1:250				

Project - Originator - Volume/System - Level/Location - Type - Discipline - Number

C86557-JNP-92-XX-DR-C-7213 P01

Document/Drawing Number

APPENDIX F: OUTLINE FLOOD RISK ASSESSMENT



■ john newton & partners

jnp group

Consulting Engineers

www.jnpgroup.co.uk

Report

Flood Risk Assessment

Project:

Hogwood Farm, Finchampstead

Client:

Legal & General Homes

Reference:

C85672-R001A

Date:

February 2018

Chesham (HQ)

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- Consulting
- Flood Risk
- Geo-Environmental
- Green Energy
- Rail
- Structural Engineering
- Transport Planning

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Leamington Spa

Sheffield

DOCUMENT CONTROL SHEET

Flood Risk Assessment



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Approved by.....
Martin Hitchcock BEng
Director

FOR AND ON BEHALF OF JNP GROUP

Date: April 18

Document Issue Record

Rev	Date	Description	Prepared	Checked	Approved
~	02/18	First Issue / Draft	JC	POF	MAH
A	04/18	Updates to site layout and Park Lane Ditch Flood Risk	JC	MAH	MAH

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