

TECHNICAL NOTE

To: Wokingham Borough Council (Planning)

From: Vector Structures Ltd.

Subject: Green Springs, Marsh Lane, Taplow, SL6 0DF (Application Reference PL/24/2580/CONDA)

Date: 27th February 2026

This technical note has been prepared to address queries regarding sustainable drainage system (SUDS) and their implementation as a result of an inspection undertaken by Wokingham Borough Council (WBC) and the Expediency Report (RFS/2022/087602) issued to the operator of the site.

This technical note addresses the key points raised in the Expediency Report and propose plans to address the points raised as a matter of urgency.

Existing Area

The site covers an area of approximately three hectares. It currently has a large, detached dwelling located down a private drive off Waterloo Road to the south. There are several single storey buildings, used in association with industrial and commercial businesses operating from the site. To the north of the site there is a large pond enclosed by dense vegetation, and a large field currently occupied other structures and some stored equipment and material.

Flood Risk Assessment

An initial review of the Flood Map for Planning for this confirms that the area is entirely within a Flood Zone 1 but nonetheless, due to the size of the area, a detailed Flood Risk Assessment will be prepared for this site. At this stage, a detailed flood map (also known as the EA Product 4) will be requested from the EA. This will enable a clear understand of the flood levels and to review the siting of any proposed building in accordance with the National Planning Policy Framework.

SUDS

As part of the proposal to address the issues raised by the Expediency Report, a detailed SUDS assessment will also be undertaken for the site. This assessment will compare the Greenfield runoff rates of the site to the current runoff rates to be generated by the total impermeable area from the developed site.

At this stage, initial information on Ground Investigation obtained for a nearby site indicates that it is unlikely than filtration can be used as a method of surface water management. It is therefore proposed that a combination of attenuation and rainwater harvesting from roof will be deployed – in accordance with the current SUDS Hierarchy.

Foul Drainage

Due to the mixed use of the site, it is unlikely that all surface or road runoff will be suitable for attenuation and discharge into nearby watercourses. It is therefore proposed that a detailed site assessment is undertaken and areas where surface water runoff might be deemed as posing a risk to quality of nearby watercourses may have to be either covered or the foul drainage system for the site.

A review of the current public sewer network system suggests there is no foul water drainage system in the area so it is likely that if additional volumes of foul drainage is generated, on site private foul water treatment systems will need to be upgraded to deal with the additional foul water generated.

Any treated wastewater from the private treatment systems will need to be to a level agreed with the EA prior to discharge into the environment.

Conclusion

A detailed Flood Risk Assessment is planned together with a detailed SUDS assessment and foul drainage management system for this site.

The client has provided topographical surveys need for the design, and a formal request has been made to the EA for the EA Product 4 for this area. All information, when received will be used to prepare a details surface water management plan for the area for onward submission to the Planning Authority for review.

Flood map for planning

Your reference	Location (easting/northing)	Created
Unspecified	483468/168092	25 February 2026 10:00

Your selected location is in flood zone 1, an area with a low probability of flooding.

You will need to do a flood risk assessment if your site is **any of the following**:

- bigger than 1 hectare (ha)
- in an area with critical drainage problems as notified by the Environment Agency
- identified as being at increased flood risk in future by the local authority's strategic flood risk assessment
- at risk from other sources of flooding (such as surface water or reservoirs) and its development would increase the vulnerability of its use (such as constructing an office on an undeveloped site or converting a shop to a dwelling)

Notes

The flood map for planning shows river and sea flooding data only. It doesn't include other sources of flooding. It is for use in development planning and flood risk assessments.

This information relates to the selected location and is not specific to any property within it. The map is updated regularly and is correct at the time of printing.

Flood risk data is covered by the Open Government Licence which sets out the terms and conditions for using government data. <https://www.nationalarchives.gov.uk/doc/open-government-licence/version/3>

Use of the address and mapping data is subject to Ordnance Survey public viewing terms under Crown copyright and database rights 2026 AC0000807064. <https://flood-map-for-planning.service.gov.uk/os-terms>



Flood map for planning

Your reference

Unspecified

Location (easting/northing)
483468/168092

Scale
1:2,500

Created
25 Feb 2026 10:01

-  Selected area
-  Flood zone 3
-  Flood zone 2
-  Flood zone 1
-  Flood defence
-  Main river
-  Water storage area


0 20 40 60m



Greenfield runoff rate estimation tool

www.uksuds.com | Greenfield runoff rate estimation tool (<https://www.uksuds.com/>)

This is an estimation of the greenfield runoff rates that are used to meet normal best practice criteria in line with Environment Agency guidance “Rainfall runoff management for developments”, SC030219 (2013), the SuDS Manual C753 (CIRIA, 2015) and the non-statutory standards for SuDS (Defra, 2015). This information on greenfield runoff rates may be the basis for setting consents for the drainage of surface water runoff from sites.

Project details

Date	27/02/2026
Calculated by	RNKA
Reference	Land at Bigwood House
Model version	2.2.3

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MORE INFO

Location

Site name	Bigwood House
Site location	Wokingham



Site easting (British National Grid)	483443
Site northing (British National Grid)	168225

Site details

Total site area (ha)	0.02	ha
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Greenfield runoff

Method

Method	IH124
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IH124

	<u>My value</u>		<u>Map value</u>	
SAAR (mm)	664	mm	☐	664
How should SPR be derived?	WRAP soil type			
WRAP soil type	4		☐	4
SPR	0.47			
QBar (IH124) (l/s)	0.1	l/s		

Growth curve factors

	<u>My value</u>		<u>Map value</u>	
Hydrological region	6		☐	6
1 year growth factor	0.85			
2 year growth factor	0.88			
10 year growth factor	1.62			
30 year growth factor	2.3			
100 year growth factor	3.19			
200 year growth factor	3.74			

Results

Method	IH124	
Flow rate 1 year (l/s)	0.1	l/s
Flow rate 2 year (l/s)	0.1	l/s
Flow rate 10 years (l/s)	0.1	l/s
Flow rate 30 years (l/s)	0.2	l/s
Flow rate 100 years (l/s)	0.3	l/s
Flow rate 200 years (l/s)	0.3	l/s

Please note runoff estimation is subject to significant uncertainty. Results are therefore normally reported to only 1 decimal place. Where 2 decimal places are provided, this does not indicate accuracy to this level, it has been adopted to prevent 'zero' figures from being reported. Outputs less than 0.01 l/s are reported as 0.01 l/s.

Disclaimer

This report was produced using the Greenfield runoff rate estimation tool (2.2.3) developed by HR Wallingford and available at uksuds.com (<https://www.uksuds.com/>). The use of this tool is subject to the UK SuDS terms and conditions and licence agreement, which can both be found at [uksuds.com/terms-conditions](https://www.uksuds.com/terms-conditions) (<https://www.uksuds.com/terms-conditions>). The outputs from this tool have been used to estimate Greenfield runoff rates. The use of these results is the responsibility of the users of this tool. No liability will be accepted by HR Wallingford, the Environment Agency, Centre for Ecology and Hydrology, Wallingford Hydrosolutions or any other organisation for the use of these data in the design or operational characteristics of any drainage scheme.

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Design Settings

Rainfall Methodology	FSR	Maximum Time of Concentration (mins)	30.00
Return Period (years)	100	Maximum Rainfall (mm/hr)	50.0
Additional Flow (%)	40	Minimum Velocity (m/s)	1.00
FSR Region	England and Wales	Connection Type	Level Soffits
M5-60 (mm)	20.000	Minimum Backdrop Height (m)	0.200
Ratio-R	0.400	Preferred Cover Depth (m)	1.200
CV	0.750	Include Intermediate Ground	x
Time of Entry (mins)	2.00	Enforce best practice design rules	x

Adoptable Manhole Type

Max Width (mm)	Diameter (mm)	Max Width (mm)	Diameter (mm)
374	1200	749	1500
499	1350	900	1800

>900 Link+900 mm

Max Depth (m)	Diameter (mm)	Max Depth (m)	Diameter (mm)
1.500	1050	99.999	1200

Nodes

Name	Area (ha)	T of E (mins)	Cover Level (m)	Diameter (mm)	Easting (m)	Northing (m)	Depth (m)
dp1	0.005	2.00	100.000	100	0.000	10.000	0.300
tank1	0.000	2.00	100.000		0.000	7.000	0.320
ic1	0.005	2.00	100.000	300	0.000	5.000	0.370
ic2			100.000	300	6.000	5.000	0.410
tank2	0.000		100.000		6.000	10.000	0.350
dp2	0.005	2.00	100.000	100	6.000	12.000	0.340
ic3	0.005	2.00	100.000	300	18.000	5.000	0.490
tank3	0.000		100.000		18.000	10.000	0.460
dp3	0.000	2.00	100.000	100	18.000	12.000	0.300
outfall chamber			100.000	1200	18.000	0.000	0.490
discharge			100.000	100	20.000	0.000	0.500

Links

Name	US Node	DS Node	Length (m)	ks (mm) / n	US IL (m)	DS IL (m)	Fall (m)	Slope (1:X)	Dia (mm)	T of C (mins)	Rain (mm/hr)
1	dp1	tank1	3.000	0.300	99.700	99.680	0.020	150.0	100	2.07	50.0
2	tank1	ic1	7.000	0.300	99.680	99.630	0.050	140.0	100	2.24	50.0
3	ic1	ic2	6.000	0.300	99.630	99.590	0.040	150.0	100	2.38	50.0
4	tank2	ic2	5.000	0.300	99.650	99.590	0.060	83.3	100	2.15	50.0
5	dp2	tank2	2.000	0.300	99.660	99.650	0.010	200.0	100	2.06	50.0

Name	Vel (m/s)	Cap (l/s)	Flow (l/s)	US Depth (m)	DS Depth (m)	Σ Area (ha)	Σ Add Inflow (l/s)	Pro Depth (mm)	Pro Velocity (m/s)
1	0.683	5.4	0.9	0.200	0.220	0.005	0.0	29	0.515
2	0.708	5.6	0.9	0.220	0.270	0.005	0.0	28	0.526
3	0.683	5.4	1.9	0.270	0.310	0.010	0.0	41	0.624
4	0.924	7.3	0.9	0.250	0.310	0.005	0.0	25	0.642
5	0.589	4.6	0.9	0.240	0.250	0.005	0.0	31	0.464

Links

Name	US Node	DS Node	Length (m)	ks (mm) / n	US IL (m)	DS IL (m)	Fall (m)	Slope (1:X)	Dia (mm)	T of C (mins)	Rain (mm/hr)
6	ic2	ic3	12.000	0.300	99.590	99.510	0.080	150.0	100	2.68	50.0
7	tank3	ic3	5.000	0.300	99.540	99.510	0.030	166.7	100	2.14	50.0
8	dp3	tank3	2.000	0.300	99.700	99.540	0.160	12.5	100	2.01	50.0
9	ic3	outfall chamber	2.000	0.300	99.540	99.510	0.030	66.7	100	2.71	50.0
10	outfall chamber	discharge	2.000	0.500	99.510	99.500	0.010	200.0	100	2.77	50.0

Name	Vel (m/s)	Cap (l/s)	Flow (l/s)	US Depth (m)	DS Depth (m)	Σ Area (ha)	Σ Add Inflow (l/s)	Pro Depth (mm)	Pro Velocity (m/s)
6	0.683	5.4	2.8	0.310	0.390	0.015	0.0	52	0.694
7	0.647	5.1	0.0	0.360	0.390	0.000	0.0	0	0.000
8	2.423	19.0	0.0	0.200	0.360	0.000	0.0	0	0.000
9	1.036	8.1	3.8	0.360	0.390	0.020	0.0	48	1.018
10	0.553	4.3	3.8	0.390	0.400	0.020	0.0	72	0.623

Pipeline Schedule

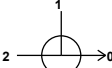
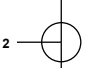
Link	Length (m)	Slope (1:X)	Dia (mm)	Link Type	US CL (m)	US IL (m)	US Depth (m)	DS CL (m)	DS IL (m)	DS Depth (m)
1	3.000	150.0	100	Circular	100.000	99.700	0.200	100.000	99.680	0.220
2	7.000	140.0	100	Circular	100.000	99.680	0.220	100.000	99.630	0.270
3	6.000	150.0	100	Circular	100.000	99.630	0.270	100.000	99.590	0.310
4	5.000	83.3	100	Circular	100.000	99.650	0.250	100.000	99.590	0.310
5	2.000	200.0	100	Circular	100.000	99.660	0.240	100.000	99.650	0.250
6	12.000	150.0	100	Circular	100.000	99.590	0.310	100.000	99.510	0.390
7	5.000	166.7	100	Circular	100.000	99.540	0.360	100.000	99.510	0.390
8	2.000	12.5	100	Circular	100.000	99.700	0.200	100.000	99.540	0.360
9	2.000	66.7	100	Circular	100.000	99.540	0.360	100.000	99.510	0.390
10	2.000	200.0	100	Circular	100.000	99.510	0.390	100.000	99.500	0.400

Link	US Node	Dia (mm)	Node Type	MH Type	DS Node	Dia (mm)	Node Type	MH Type
1	dp1	100	Manhole	Adoptable	tank1		Junction	
2	tank1		Junction		ic1	300	Manhole	Adoptable
3	ic1	300	Manhole	Adoptable	ic2	300	Manhole	Adoptable
4	tank2		Junction		ic2	300	Manhole	Adoptable
5	dp2	100	Manhole	Adoptable	tank2		Junction	
6	ic2	300	Manhole	Adoptable	ic3	300	Manhole	Adoptable
7	tank3		Junction		ic3	300	Manhole	Adoptable
8	dp3	100	Manhole	Adoptable	tank3		Junction	
9	ic3	300	Manhole	Adoptable	outfall chamber	1200	Manhole	Adoptable
10	outfall chamber	1200	Manhole	Adoptable	discharge	100	Manhole	Adoptable

Manhole Schedule

Node	Easting (m)	Northing (m)	CL (m)	Depth (m)	Dia (mm)	Connections	Link	IL (m)	Dia (mm)	
dp1	0.000	10.000	100.000	0.300	100		0	1	99.700	100

Manhole Schedule

Node	Easting (m)	Northing (m)	CL (m)	Depth (m)	Dia (mm)	Connections	Link	IL (m)	Dia (mm)
tank1	0.000	7.000	100.000	0.320			1	99.680	100
ic1	0.000	5.000	100.000	0.370	300		2	99.630	100
ic2	6.000	5.000	100.000	0.410	300		3	99.590	100
tank2	6.000	10.000	100.000	0.350			4	99.650	100
dp2	6.000	12.000	100.000	0.340	100		5	99.660	100
ic3	18.000	5.000	100.000	0.490	300		6	99.510	100
tank3	18.000	10.000	100.000	0.460			7	99.540	100
dp3	18.000	12.000	100.000	0.300	100		8	99.700	100
outfall chamber	18.000	0.000	100.000	0.490	1200		9	99.510	100
discharge	20.000	0.000	100.000	0.500	100		10	99.500	100

Simulation Settings

Rainfall Methodology	FSR	Analysis Speed	Normal
FSR Region	England and Wales	Skip Steady State	x
M5-60 (mm)	20.000	Drain Down Time (mins)	240
Ratio-R	0.400	Additional Storage (m³/ha)	20.0
Summer CV	0.750	Check Discharge Rate(s)	x
Winter CV	0.840	Check Discharge Volume	x

Storm Durations

30 | 60 | 120 | 180 | 240 | 360 | 480 | 600 | 720

Return Period (years)	Climate Change (CC %)	Additional Area (A %)	Additional Flow (Q %)
1	0	0	0
30	0	0	0
100	0	0	0
100	40	0	0

Node outfall chamber Online Hydro-Brake® Control

Flap Valve	x	Objective	(HE) Minimise upstream storage
Replaces Downstream Link	✓	Sump Available	✓
Invert Level (m)	99.510	Product Number	CTL-SHE-0054-1000-0490-1000
Design Depth (m)	0.490	Min Outlet Diameter (m)	0.075
Design Flow (l/s)	1.0	Min Node Diameter (mm)	1200

Node tank1 Depth/Area Storage Structure

Base Inf Coefficient (m/hr)	0.00000	Safety Factor	2.0	Invert Level (m)	99.680
Side Inf Coefficient (m/hr)	0.00000	Porosity	1.00	Time to half empty (mins)	96

Depth (m)	Area (m²)	Inf Area (m²)	Depth (m)	Area (m²)	Inf Area (m²)	Depth (m)	Area (m²)	Inf Area (m²)
0.000	4.5	0.0	0.300	4.5	0.0	0.301	0.0	0.0

Node tank2 Depth/Area Storage Structure

Base Inf Coefficient (m/hr)	0.00000	Safety Factor	2.0	Invert Level (m)	99.650
Side Inf Coefficient (m/hr)	0.00000	Porosity	1.00	Time to half empty (mins)	113

Depth (m)	Area (m²)	Inf Area (m²)	Depth (m)	Area (m²)	Inf Area (m²)	Depth (m)	Area (m²)	Inf Area (m²)
0.000	4.5	0.0	0.300	4.6	0.0	0.301	0.0	0.0

Node tank3 Depth/Area Storage Structure

Base Inf Coefficient (m/hr)	0.00000	Safety Factor	2.0	Invert Level (m)	99.540
Side Inf Coefficient (m/hr)	0.00000	Porosity	1.00	Time to half empty (mins)	132

Depth (m)	Area (m²)	Inf Area (m²)	Depth (m)	Area (m²)	Inf Area (m²)	Depth (m)	Area (m²)	Inf Area (m²)
0.000	3.0	0.0	0.300	3.0	0.0	0.301	0.0	0.0

Approval Settings

Node Size	✓	Full Bore Velocity	✓
Node Losses	✓	Minimum Full Bore Velocity (m/s)	1.000
Link Size	✓	Maximum Full Bore Velocity (m/s)	3.000
Minimum Diameter (mm)	150	Proportional Velocity	✓
Link Length	✓	Return Period (years)	100
Maximum Length (m)	100.000	Minimum Proportional Velocity (m/s)	0.750
Coordinates	✓	Maximum Proportional Velocity (m/s)	3.000
Accuracy (m)	1.000	Surcharged Depth	✓
Crossings	✓	Return Period (years)	100
Cover Depth	x	Maximum Surcharged Depth (m)	0.100
Backdrops	✓	Flooding	✓
Minimum Backdrop Height (m)	0.200	Return Period (years)	30
Maximum Backdrop Height (m)	1.500	Time to Half Empty	x



Approval Settings

Discharge Rates	✓	100 year 360 minute (m ³)
Discharge Volume	✓	

Results for 1 year Critical Storm Duration. Lowest mass balance: 99.79%

Node Event		Peak (mins)	Depth (m)	Inflow (l/s)	Flood (m ³)		
30 minute summer		16	0.024	0.7	0.0000		
30 minute summer		18	0.021	0.7	0.0000		
30 minute summer		17	0.032	1.1	0.0000		
30 minute winter		23	0.046	1.7	0.0000		
30 minute summer		18	0.019	0.7	0.0000		
30 minute summer		16	0.025	0.7	0.0000		
30 minute winter		23	0.124	2.2	0.0000		
30 minute winter		23	0.094	0.7	0.0000		
30 minute summer		1	0.000	0.0	0.0000		
30 minute winter		23	0.123	1.3	0.0000		
30 minute summer		1	0.000	1.0	0.0000		
Link Event (Outflow)	Link	DS Node	Outflow (l/s)	Velocity (m/s)	Flow/Cap	Link Vol (m ³)	Discharge Vol (m ³)
30 minute summer	1	tank1	0.7	0.555	0.124	0.0038	
30 minute summer	2	ic1	0.6	0.351	0.100	0.0118	
30 minute summer	3	ic2	1.1	0.475	0.213	0.0145	
30 minute summer	6	ic3	1.7	0.366	0.316	0.0617	
30 minute summer	4	ic2	0.6	0.319	0.079	0.0094	
30 minute summer	5	tank2	0.7	0.553	0.144	0.0025	
120 minute summer	9	outfall chamber	2.0	0.452	0.247	0.0097	
30 minute summer	7	ic3	-0.8	-0.164	-0.157	0.0357	
30 minute summer	8	tank3	0.0	0.000	0.000	0.0065	
30 minute winter	Hydro-Brake®	discharge	1.0				1.7

Results for 30 year Critical Storm Duration. Lowest mass balance: 99.79%

Node Event		Peak (mins)	Depth (m)	Inflow (l/s)	Flood (m ³)		
30 minute winter		28	0.062	1.4	0.0000		
30 minute winter		29	0.082	1.4	0.0000		
30 minute winter		28	0.132	2.7	0.0000		
30 minute winter		29	0.172	3.5	0.0000		
30 minute winter		29	0.112	1.9	0.0000		
30 minute winter		29	0.102	1.4	0.0000		
30 minute winter		29	0.250	3.7	0.0000		
30 minute winter		29	0.220	1.6	0.0000		
30 minute winter		29	0.060	0.2	0.0000		
30 minute winter		29	0.249	1.6	0.0000		
30 minute summer		1	0.000	1.0	0.0000		

Link Event (Outflow)	Link	DS Node	Outflow (l/s)	Velocity (m/s)	Flow/Cap	Link Vol (m ³)	Discharge Vol (m ³)
30 minute summer	1	tank1	1.7	0.684	0.325	0.0117	
30 minute summer	2	ic1	1.6	0.395	0.287	0.0446	
30 minute summer	3	ic2	2.8	0.577	0.522	0.0469	
30 minute summer	6	ic3	2.9	0.418	0.533	0.0939	
30 minute summer	4	ic2	1.5	0.381	0.203	0.0380	
30 minute summer	5	tank2	1.8	0.690	0.379	0.0141	
240 minute summer	9	outfall chamber	2.1	0.368	0.259	0.0156	
30 minute summer	7	ic3	-2.1	-0.267	-0.405	0.0391	
30 minute summer	8	tank3	-0.4	-0.087	-0.023	0.0105	
30 minute summer	Hydro-Brake®	discharge	1.0				3.6

Results for 100 year Critical Storm Duration. Lowest mass balance: 99.79%

Node Event	Peak (mins)	Depth (m)	Inflow (l/s)	Flood (m ³)
60 minute winter	48	0.128	1.2	0.0000
60 minute winter	48	0.148	1.2	0.0000
60 minute winter	48	0.198	2.1	0.0000
60 minute winter	48	0.237	2.4	0.0000
60 minute winter	48	0.177	1.3	0.0000
60 minute winter	48	0.167	1.2	0.0000
60 minute winter	49	0.316	2.7	0.0000
60 minute winter	48	0.286	1.0	0.0000
60 minute winter	48	0.126	0.2	0.0000
60 minute winter	49	0.315	1.8	0.0000
30 minute summer	1	0.000	1.0	0.0000

Link Event (Outflow)	Link	DS Node	Outflow (l/s)	Velocity (m/s)	Flow/Cap	Link Vol (m ³)	Discharge Vol (m ³)
30 minute summer	1	tank1	2.3	0.725	0.432	0.0232	
30 minute winter	2	ic1	1.5	0.383	0.272	0.0548	
30 minute summer	3	ic2	3.1	0.564	0.585	0.0469	
30 minute summer	6	ic3	2.6	0.381	0.483	0.0939	
30 minute summer	4	ic2	1.4	0.371	0.193	0.0391	
30 minute summer	5	tank2	2.0	0.731	0.435	0.0156	
180 minute winter	9	outfall chamber	2.1	0.372	0.262	0.0156	
30 minute summer	7	ic3	-2.0	-0.259	-0.398	0.0391	
30 minute winter	8	tank3	-0.5	-0.102	-0.028	0.0156	
30 minute summer	Hydro-Brake®	discharge	1.0				4.8

Results for 100 year +40% CC Critical Storm Duration. Lowest mass balance: 99.79%

Node Event		Peak (mins)	Depth (m)	Inflow (l/s)	Flood (m ³)		
60 minute winter		58	0.274	1.7	0.0000		
60 minute winter		58	0.294	1.6	0.0000		
60 minute winter		58	0.343	2.2	0.0000		
60 minute winter		57	0.383	2.3	0.0000		
60 minute winter		57	0.323	1.6	0.0000		
60 minute winter		57	0.313	1.7	0.0000		
60 minute winter		58	0.461	2.8	0.0000		
60 minute winter		58	0.432	1.2	0.0000		
60 minute winter		58	0.271	0.4	0.0000		
60 minute winter		58	0.461	1.7	0.0000		
30 minute summer		1	0.000	1.0	0.0000		

Link Event (Outflow)	Link	DS Node	Outflow (l/s)	Velocity (m/s)	Flow/Cap	Link Vol (m ³)	Discharge Vol (m ³)
30 minute summer	1	tank1	2.8	0.739	0.523	0.0235	
30 minute winter	2	ic1	1.5	0.385	0.270	0.0548	
30 minute summer	3	ic2	3.0	0.531	0.567	0.0469	
30 minute summer	6	ic3	2.4	0.350	0.452	0.0939	
30 minute winter	4	ic2	1.2	0.358	0.171	0.0391	
30 minute summer	5	tank2	2.5	0.717	0.539	0.0156	
240 minute summer	9	outfall chamber	2.1	0.473	0.262	0.0156	
30 minute summer	7	ic3	-2.5	-0.315	-0.485	0.0391	
30 minute summer	8	tank3	-0.7	-0.105	-0.035	0.0156	
30 minute summer	Hydro-Brake®	discharge	1.0				6.7